



Lateral Response of Cold-formed Steel Framed Steel Sheathed In-line Wall Systems Detailed For Mid-rise Buildings

Amanpreet Singh | University of California San Diego

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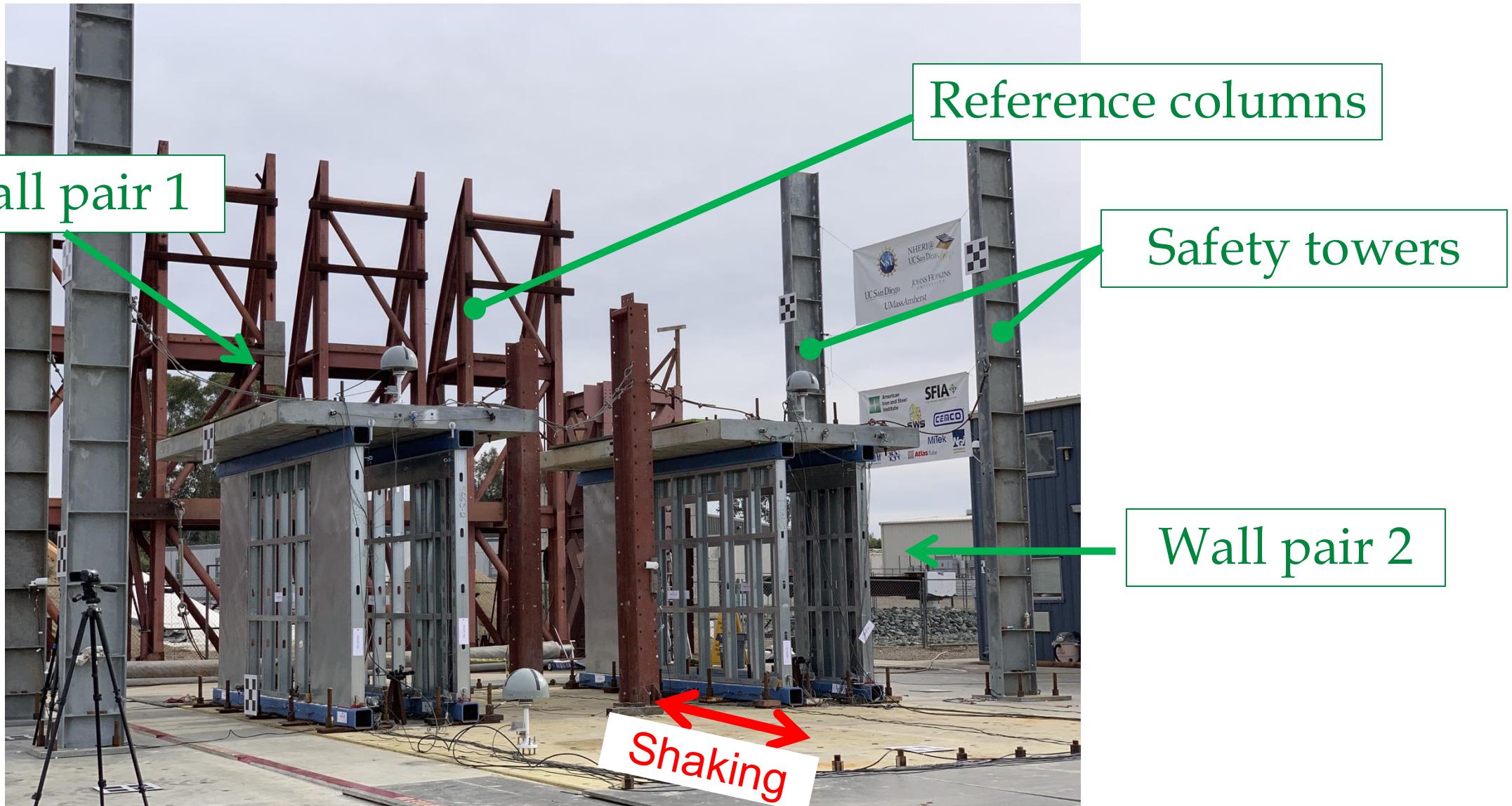
Wall-Line Tests: *Experiment Objectives*

- Characterize dynamic performance of CFS framed walls subjected to in-line earthquake motions
- *Effect of finishes* and *effects of openings* on wall behavior
- *Comparison of Type-I and Type-II* walls
- Compare *steel tension tie-rods* assembly versus *holdown* systems
- Compare *symmetrical and unsymmetrical* walls
- Examine lateral load sharing between shear walls placed in-line with gravity walls

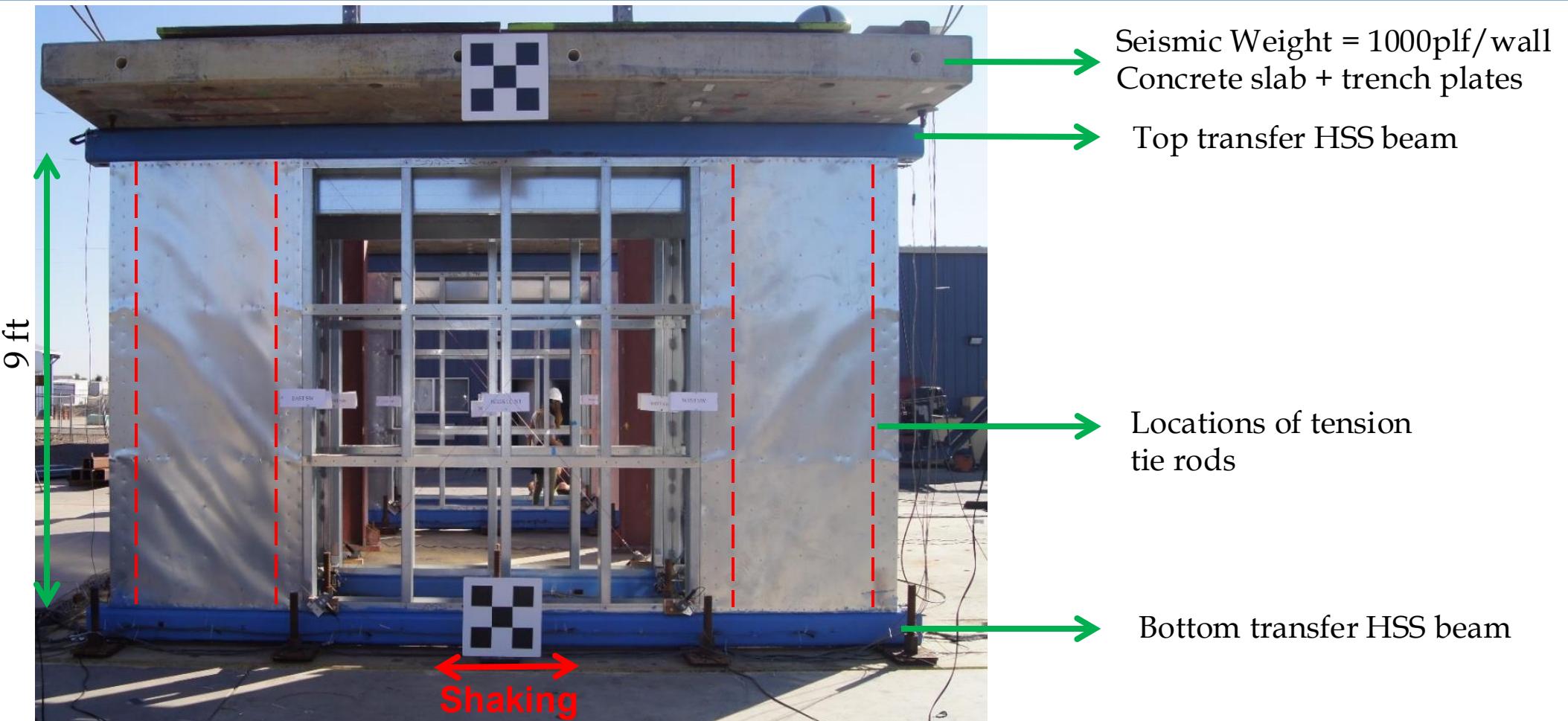
In total, **16 unique configurations**; blend of **dynamic** (shake table) and **quasi-static reversed cyclic** (displacement control) testing regimes



Test Setup: *Shake table tests (NHERI@UCSD)*



Test Setup: SGGS-1 specimen (baseline)



4 ft 4 ft 4 ft 4 ft

Shear Gravity Gravity Shear

-1 (Type I)

Naming convention

Test Protocol: *Shake table tests*

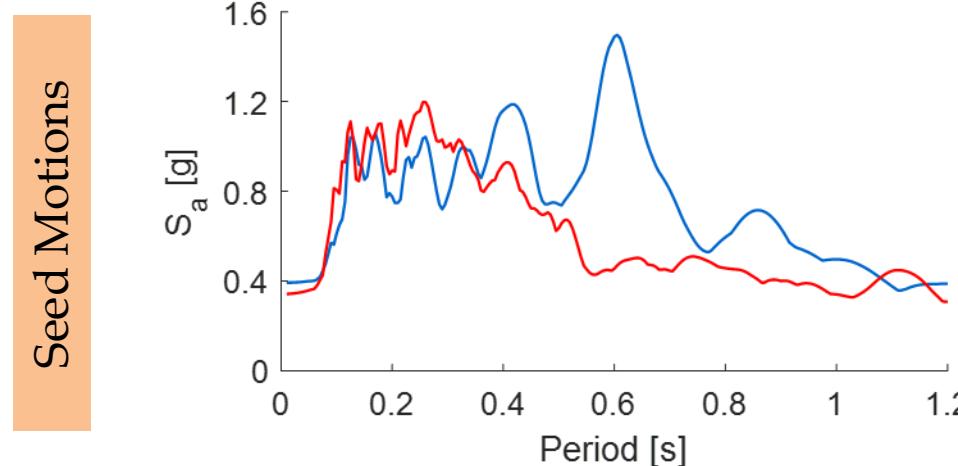
- Scaled ground motions (increasing intensity)

1. Elastic Level

- 1994 Northridge - Canoga Park
- 2010 Maule, Chile - Curico

2. Quasi-elastic Level

- 1994 Northridge - Canoga Park

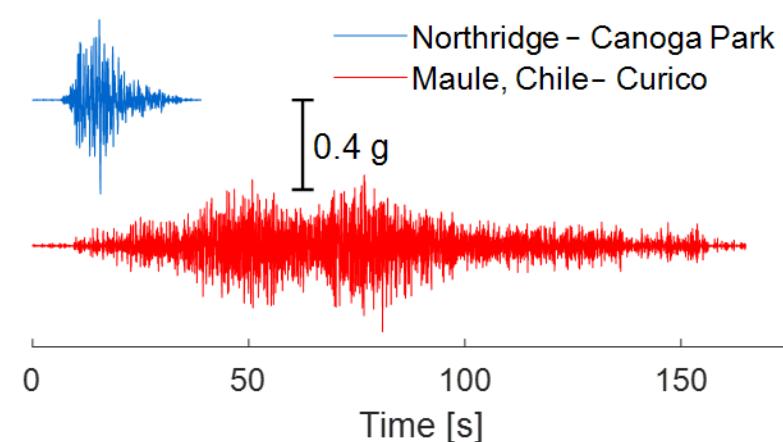


3. Design Level

- 1994 Northridge - Canoga Park

4. Above Design Level (optional)

- 1994 Northridge - Canoga Park

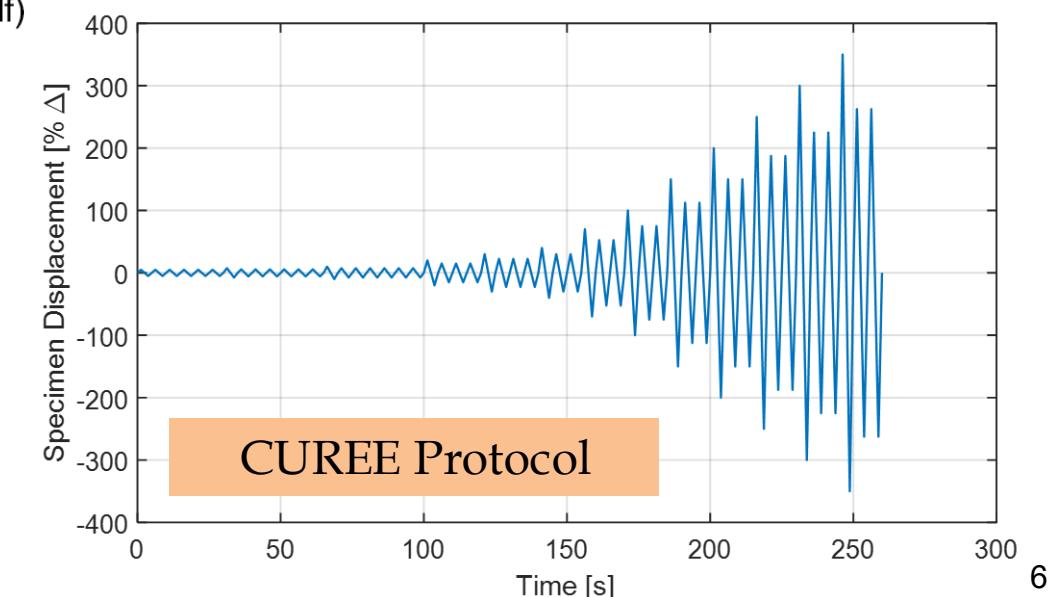
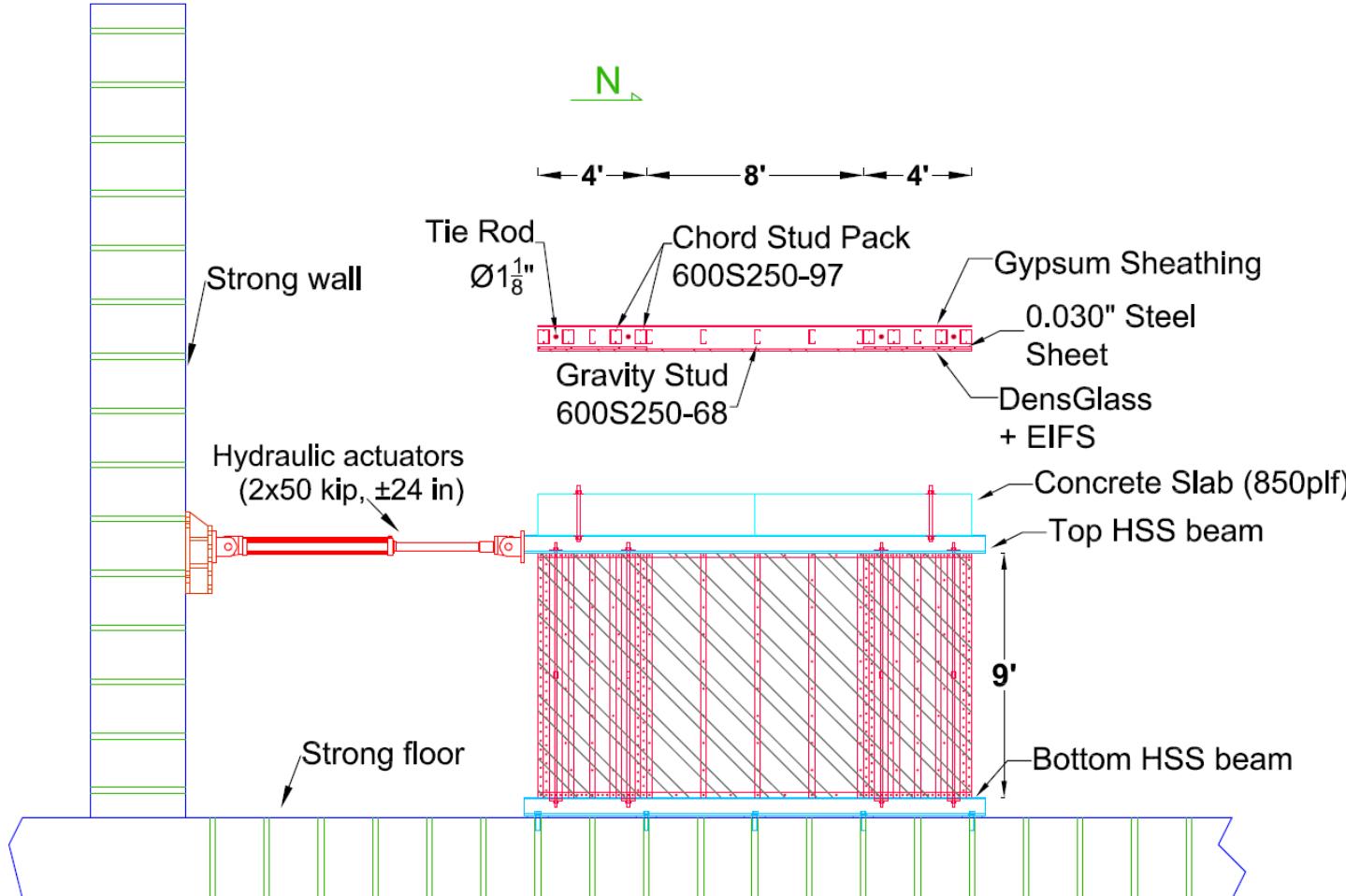


- Low-amplitude white-noise base excitation tests

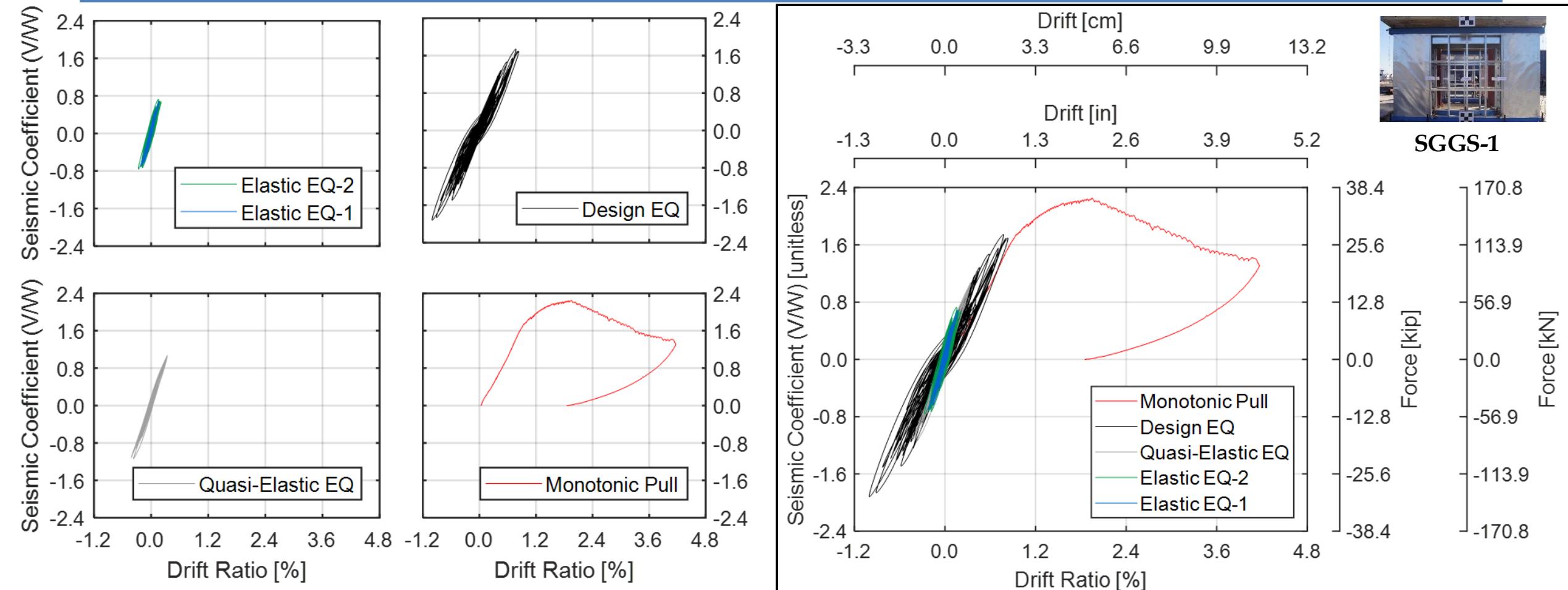
- Before & after each EQ tests (duration: 4 minutes)
- Amplitude: 1.5% g & 3% g RMS

- Static monotonic pull over for post-peak behavior (for select specimens)

Test Setup and Protocol: *Quasi-static tests*

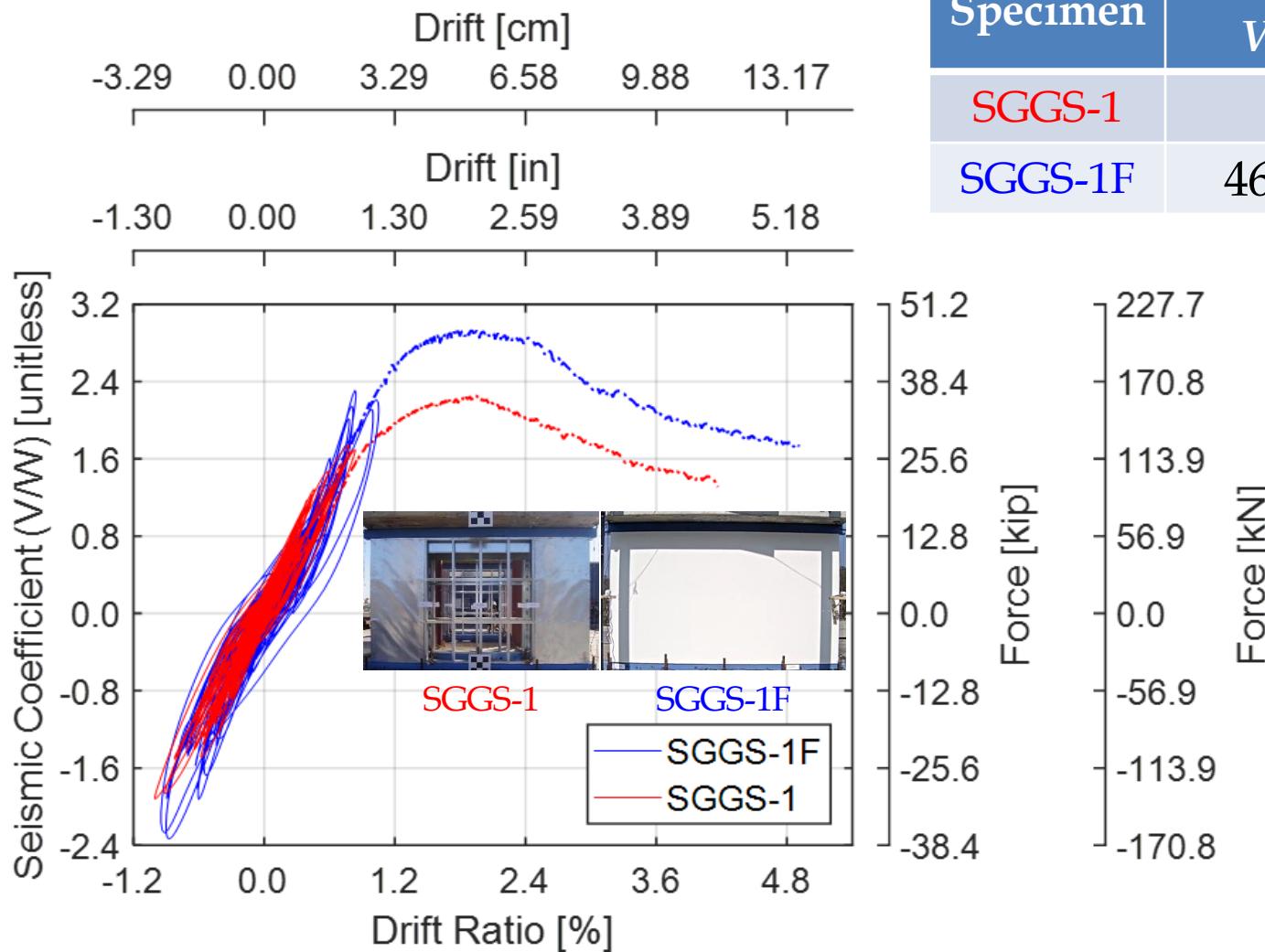


Force-Displacement Response: SGGS-1 (baseline)



Specimen	Strength, V_{max} [kip]	Drift, $\delta_{V_{max}}$ [in] (%)	Initial Stiffness, k_e [kip/in]	Secant Stiffness, k_{sec} [kip/in]
SGGS-1	36.0	2.11 (1.95%)	47.4	17.1

Comparison: Finished vs Unfinished



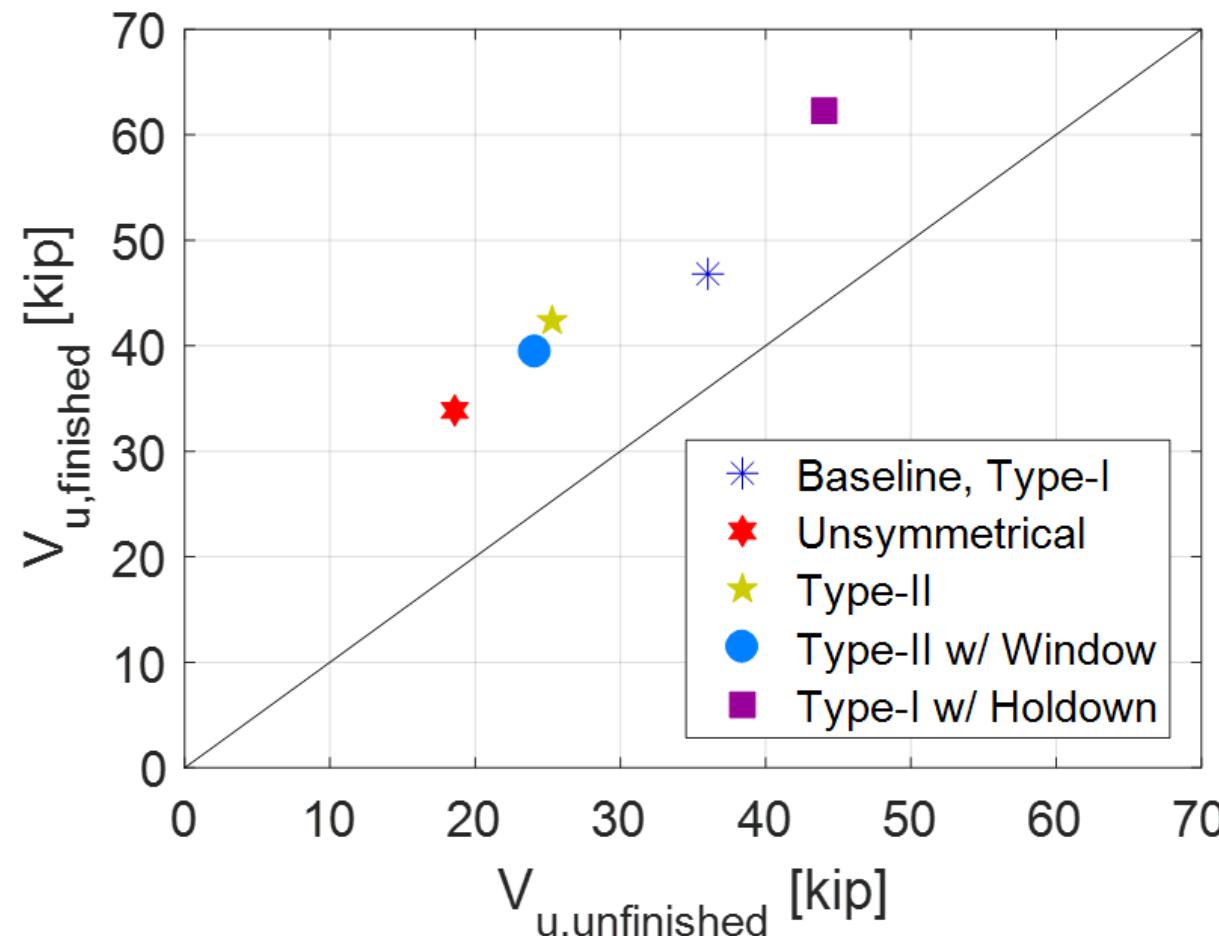
Specimen	Peak Strength, V_{max} [kip]	Drift, $\delta_{V_{max}}$ [in] (%)	Initial Stiffness, k_e [kip/in]
SGGS-1	36.0	2.11 (1.95%)	47.4
SGGS-1F	46.8 ($\uparrow 30\%$)	2.05 (1.90%)	116.8 ($\uparrow 150\%$)

- **Strength** increase
- **Initial stiffness** increase
- No negative effect on **drift at strength**

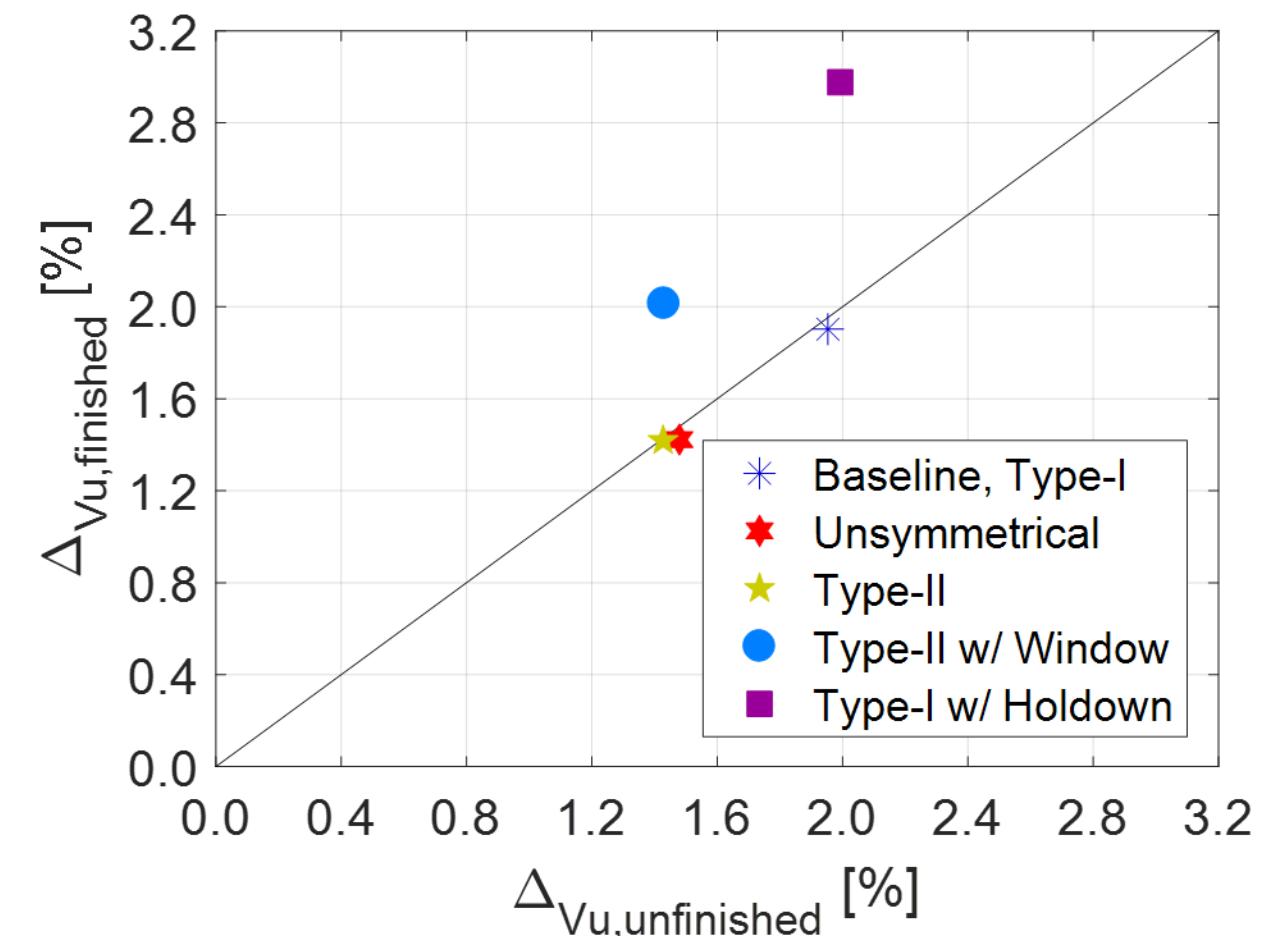
Unfinished = Wall framing

Finished = Exterior Insulation Finish System (EIFS) and gypsum panels on interior face on wall framing

Effect of Finish Application

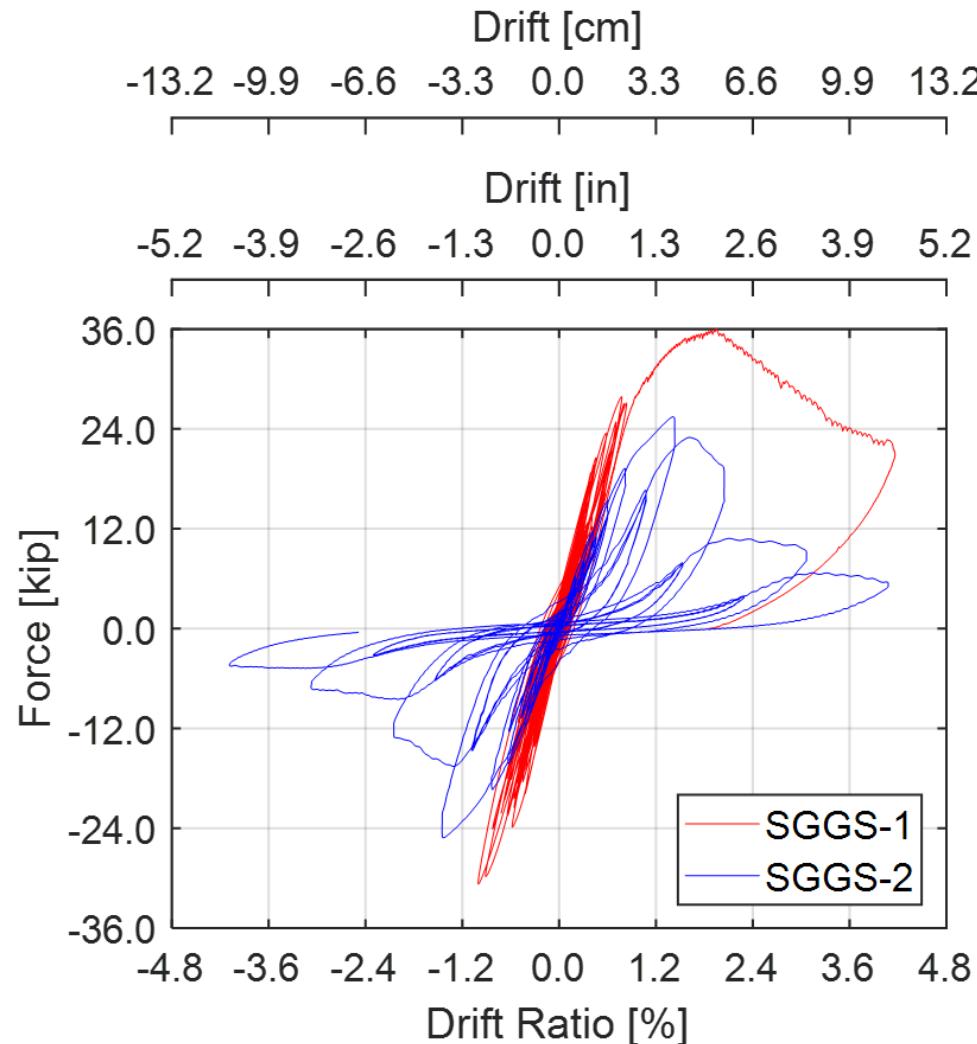


$$V_{u,\text{finished}} = 1.01 \times V_{u,\text{unfinished}} + 930 \text{ plf}$$



- **Strength** increase (additive strength model)
- No negative effect on **drift at strength**

Comparison: Type-I vs Type-II Wall Systems



Specimen	Strength, V_{max} [kip]	Drift, $\delta_{V_{max}}$ [in] (%)	Initial Stiffness*, k_e [kip/in]
SGGS-1	36.0	2.11 (1.95%)	47.4
SGGS-2	25.5 ($\downarrow 30\%$)	1.53 (1.41%)	25.9 ($\downarrow 45\%$)

- **Strength** decrease, but not 50% as suggested by code
- **Initial stiffness** decrease
- Lower **drift at strength**



SGGS-1



SGGS-2

Type-I = anchorage at each end of wall segment
 Type-II = anchorage at ends of wall

Concluding Remarks: Observations

- Symmetry: Unsymmetrical wall with one 4ft shear wall segment
 - $\sim 50\%$  in strength and stiffness
 - Aligning with conceptual design perspective
- Window Opening & Window Framing:
 - Negligible effect on strength or stiffness
 - Drift capacity not effected
 - Damage to adjacent stud packs concerning
- Tension Tie-Rod/Holdown detailing:
 - Strength $\pm 20\%-30\%$ with holdowns
 - Similar initial stiffness
 - Tension rods offer advantages: easy installation and continuous floor-to-floor system
- Finishes: EIFS and Gypsum boards
 - Strength $\pm 30\%-80\% = 930\text{ plf}$
 - Initial stiffness $\pm 1.5x\text{-}3x$
 - Period elongation $< 10\%$ (QE)
 - Damping $\sim \pm 50\%$
 - No derogatory effect on drift capacity
- Anchorage Detailing: Type-II wall
 - $\sim 30\%-35\%$  in strength, not 50% as suggested by code
 - Initial stiffness  $30\%-45\%$ with tie-rods anchorage at ends of wall



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