



# Lateral Response of Cold-formed Steel Framed Steel Sheathed In-line Wall Systems Detailed For Mid-rise Buildings

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*CFSRC Summer Symposium, May 26-27, 2020*

UC San Diego JOHNS HOPKINS  
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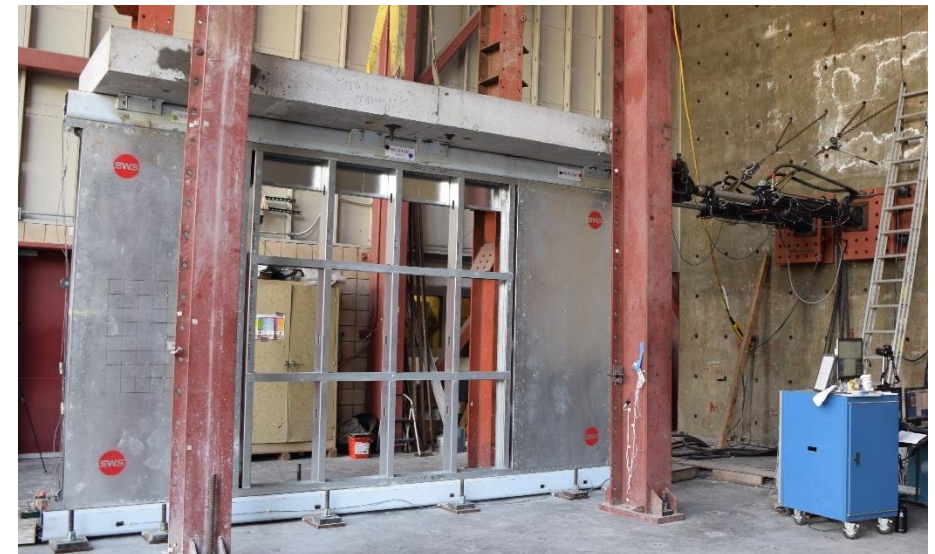
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# Wall-Line Tests: *Experiment Objectives*

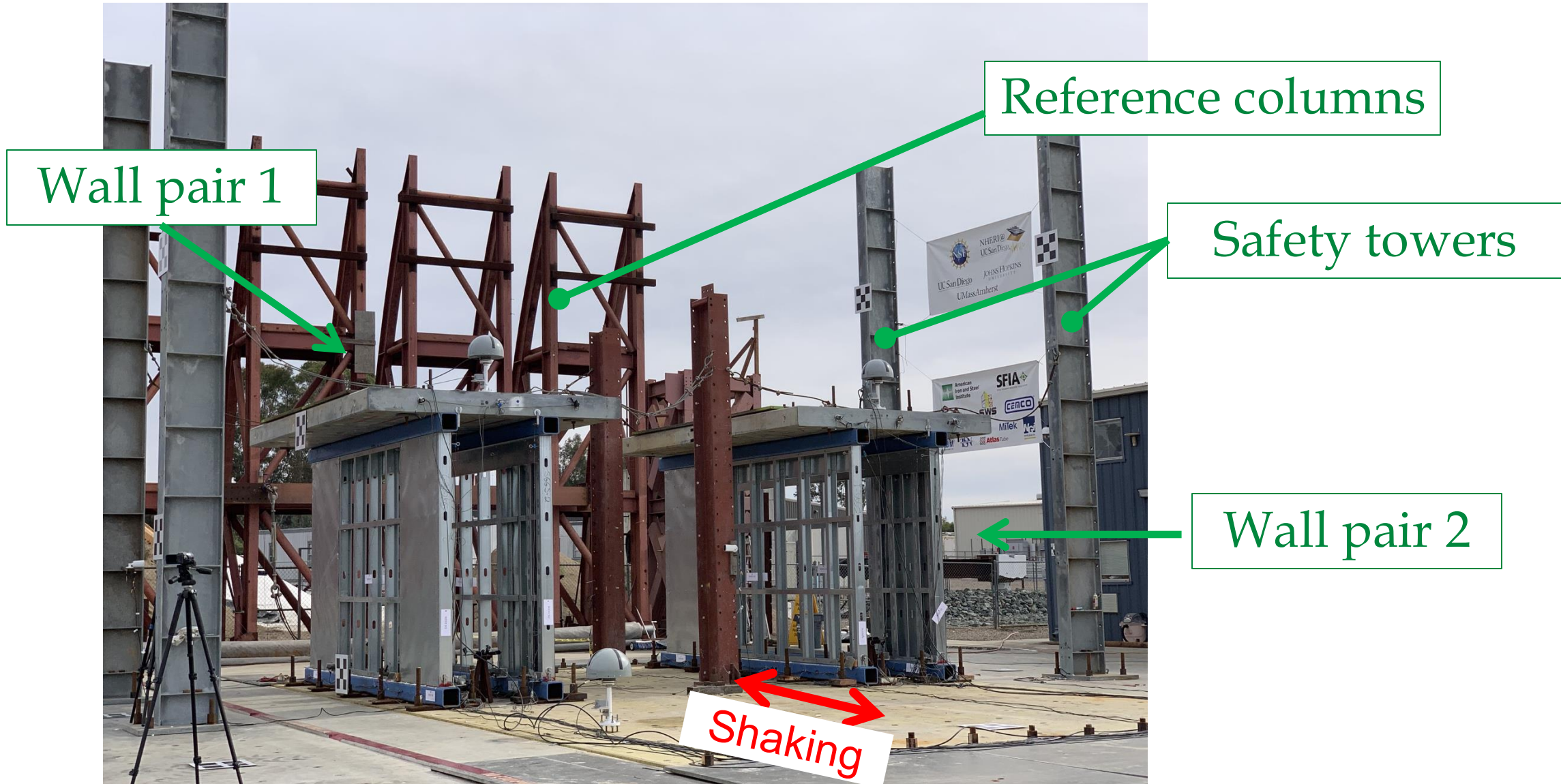
- Characterize dynamic performance of CFS framed walls subjected to in-line earthquake motions
- Effect of finishes and effects of openings on wall behavior
- Comparison of Type-I and Type-II walls
- Compare steel tension tie-rods assembly versus holdown systems
- Compare symmetrical and unsymmetrical walls
- Examine lateral load sharing between shear walls placed in-line with gravity walls

In total, **16 unique configurations**; blend of **dynamic** (shake table) and **quasi-static reversed cyclic** (displacement control) testing regimes

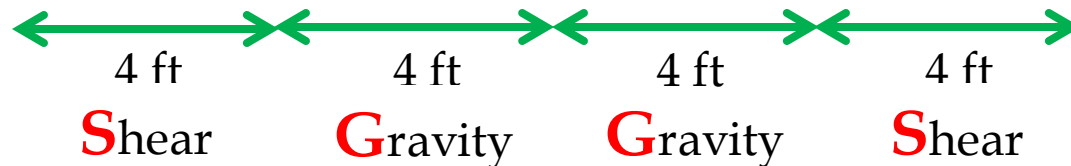
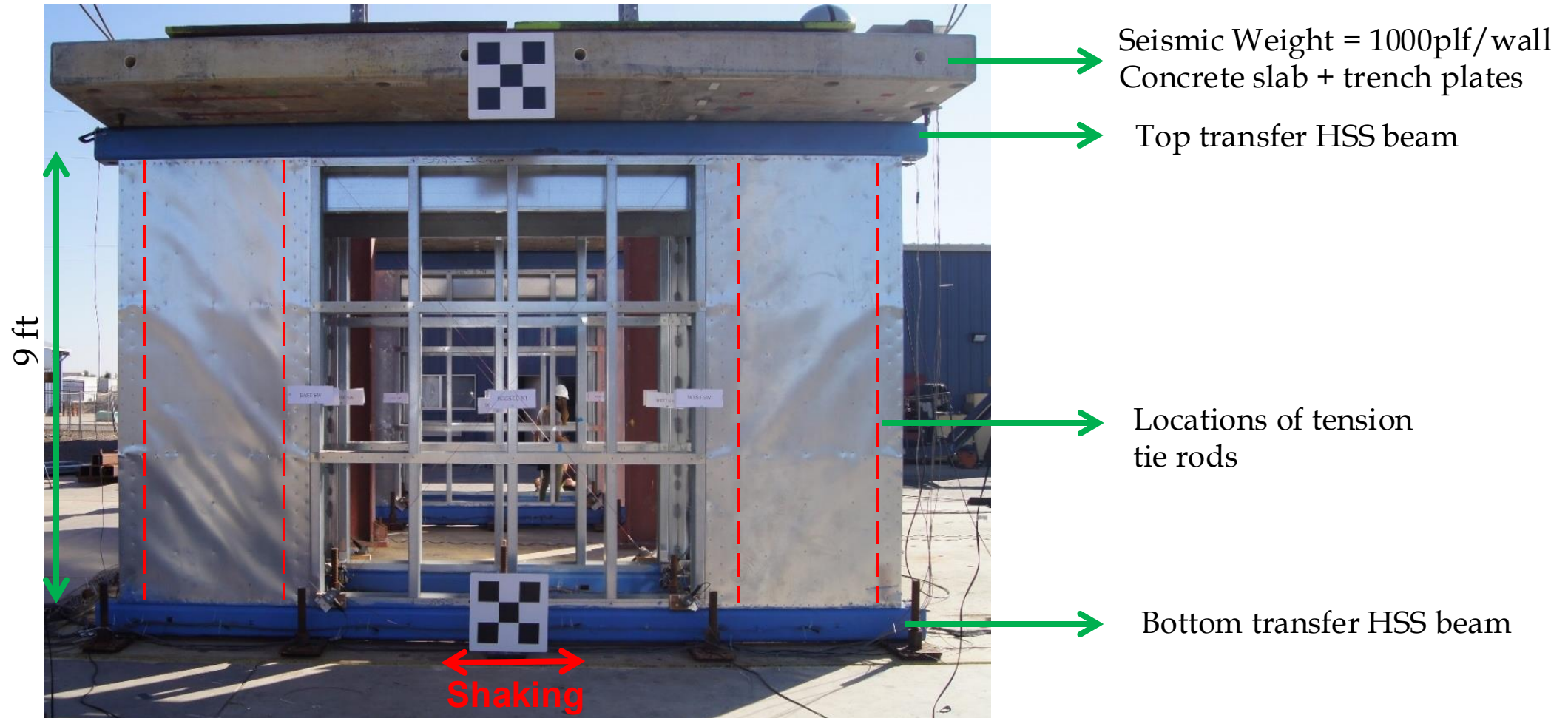




# Test Setup: *Shake table tests (NHERI@UCSD)*



# Test Setup: *SGGS-1 specimen (baseline)*



**-1** (Type I)

Naming convention

# Test Protocol: *Shake table tests*

- Scaled ground motions (increasing intensity)

1. Elastic Level

- 1994 Northridge – Canoga Park
- 2010 Maule, Chile – Curico

2. Quasi-elastic Level

- 1994 Northridge – Canoga Park

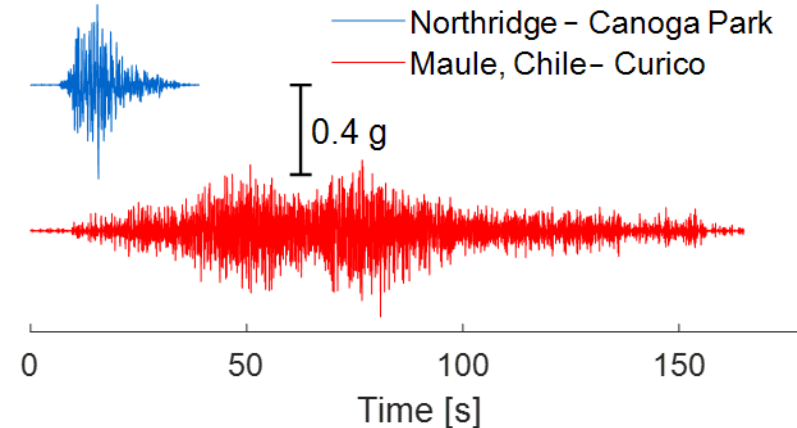
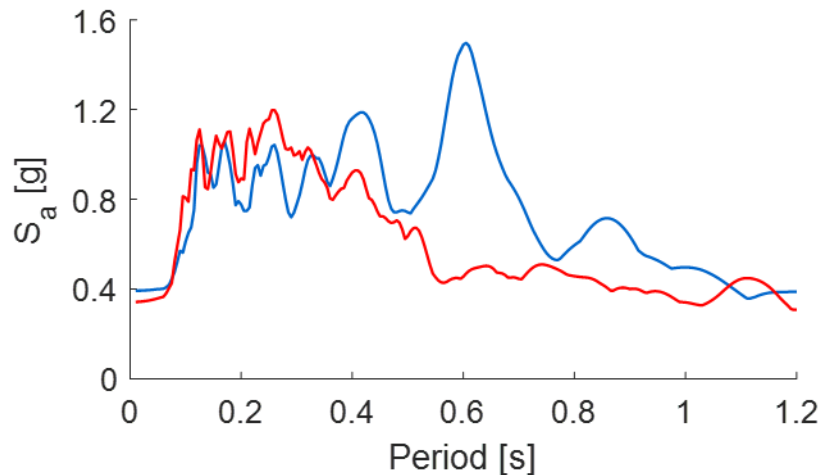
3. Design Level

- 1994 Northridge – Canoga Park

4. Above Design Level (optional)

- 1994 Northridge – Canoga Park

Seed Motions



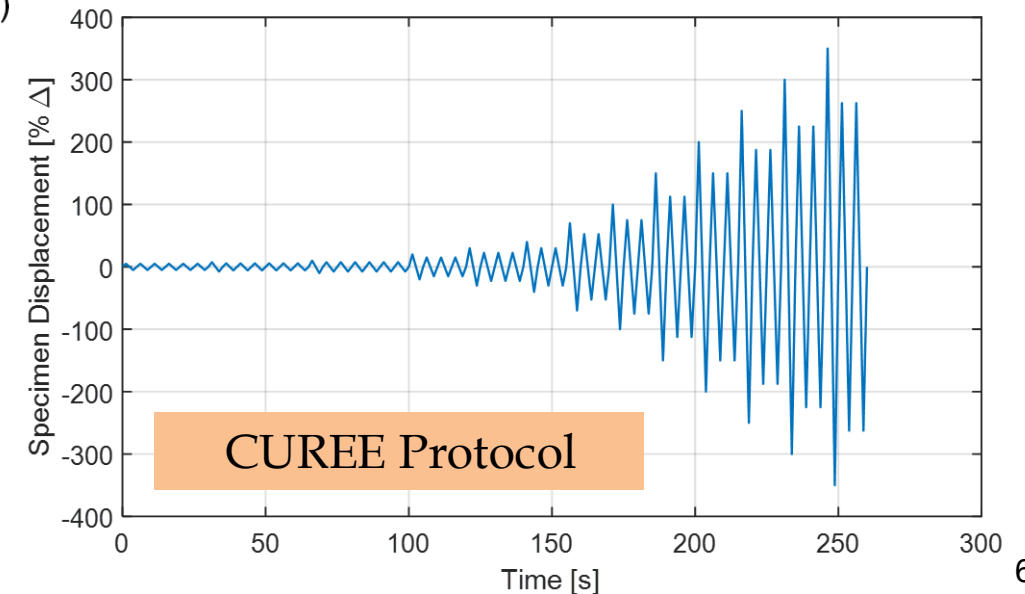
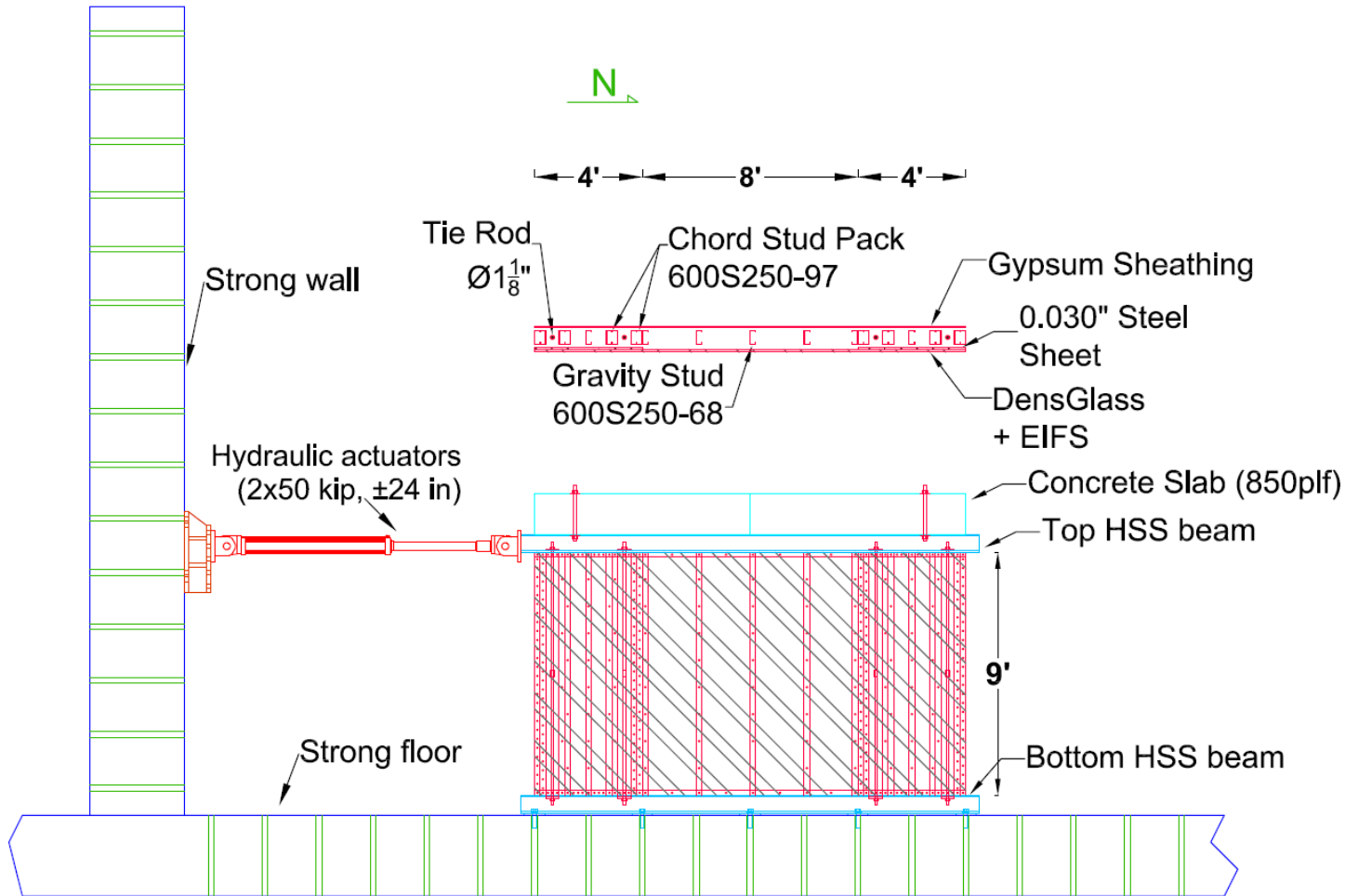
- Low-amplitude white-noise base excitation tests

- Before & after each EQ tests (duration: 4 minutes)
- Amplitude: 1.5% g & 3% g RMS

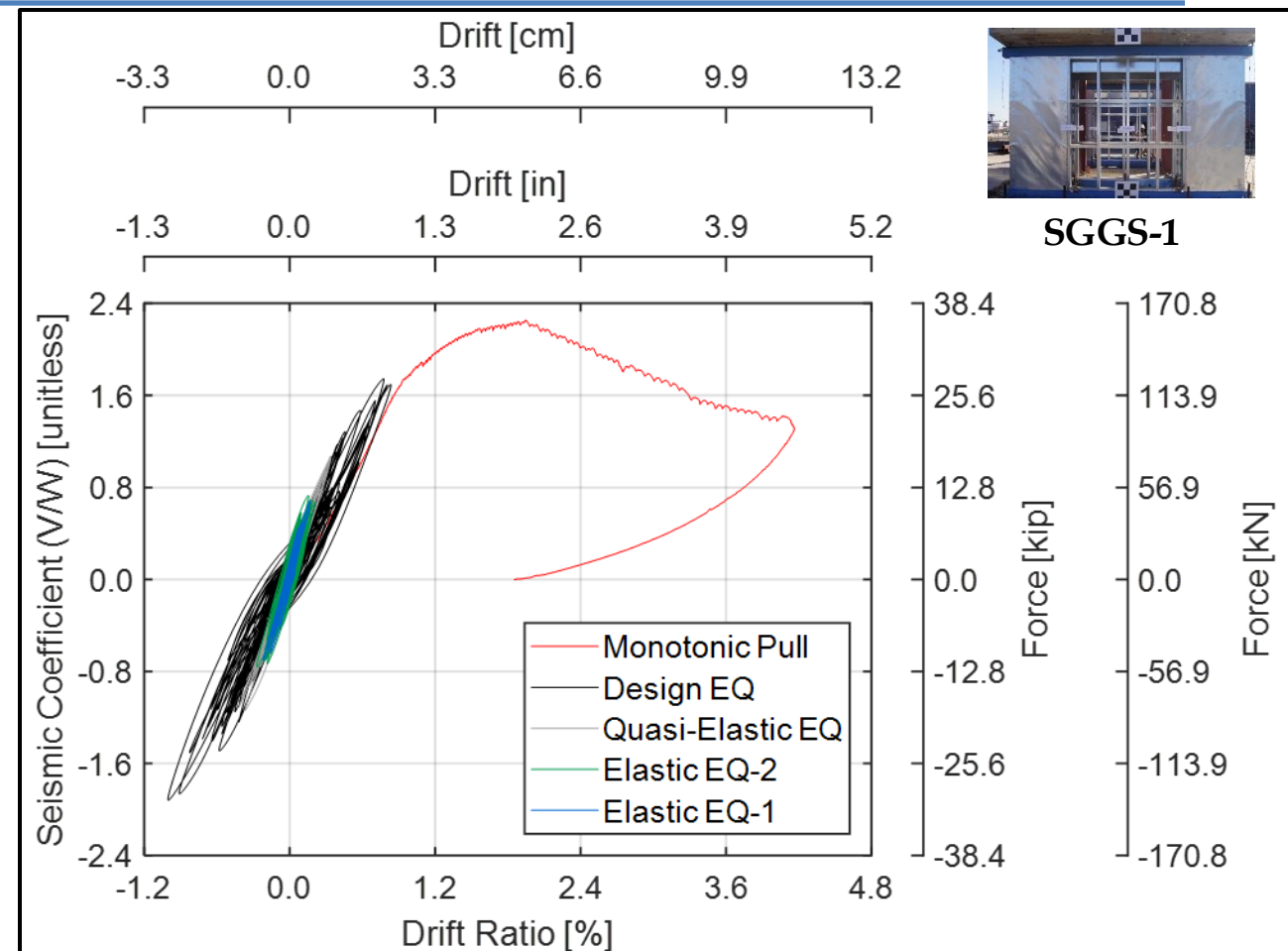
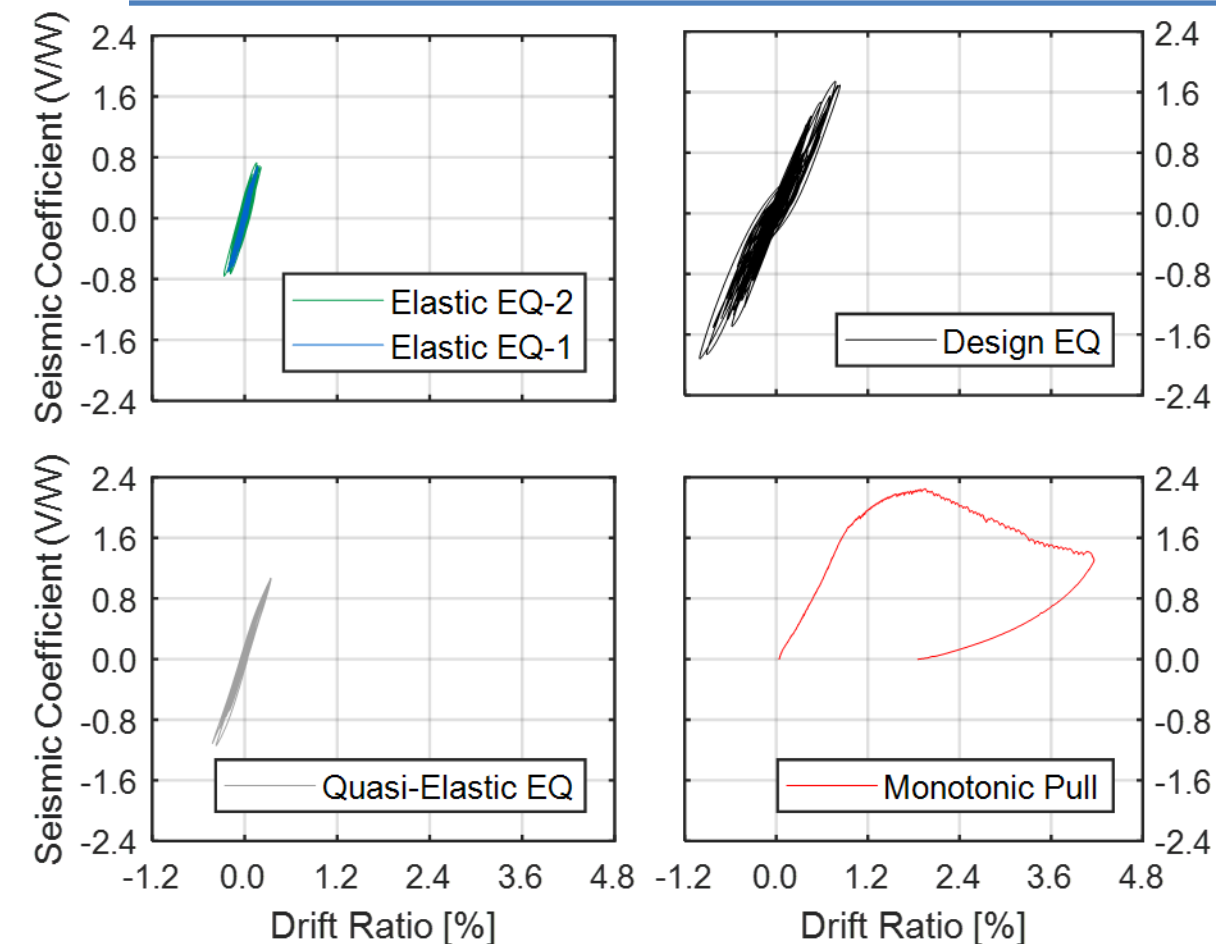
- Static monotonic pull over for post-peak behavior (for select specimens)



# Test Setup and Protocol: *Quasi-static tests*



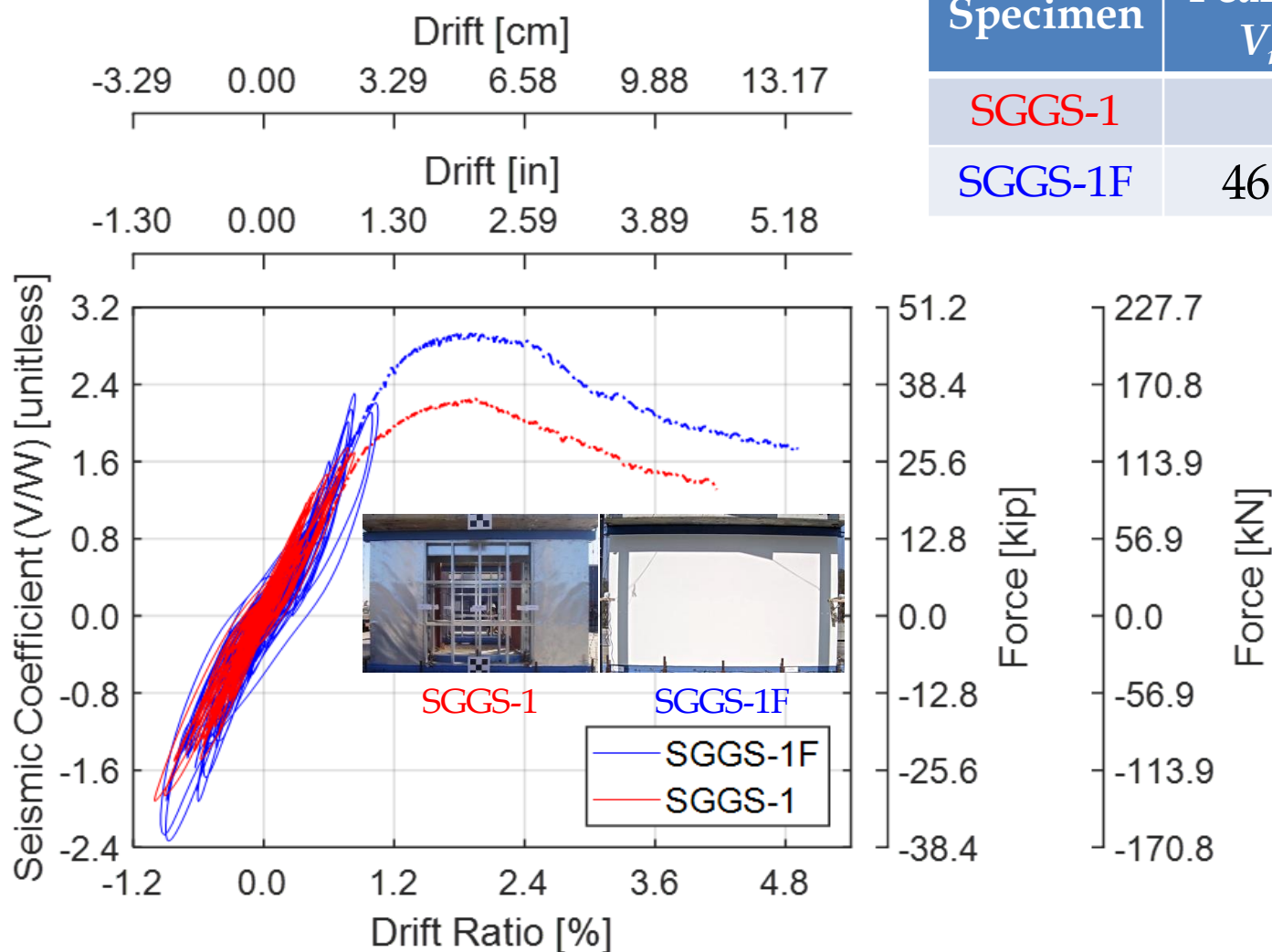
# Force-Displacement Response: *SGGS-1 (baseline)*



| Specimen | Strength,<br>$V_{max}$ [kip] | Drift,<br>$\delta_{V_{max}}$ [in] (%) | Initial Stiffness,<br>$k_e$ [kip/in] | Secant Stiffness,<br>$k_{sec}$ [kip/in] |
|----------|------------------------------|---------------------------------------|--------------------------------------|---|
| SGGS-1   | 36.0                         | 2.11 (1.95%)                          | 47.4                                 | 17.1                                    |

# Comparison: Finished vs Unfinished

| Specimen | Peak Strength,<br>$V_{max}$ [kip] | Drift,<br>$\delta_{V_{max}}$ [in] (%) | Initial Stiffness,<br>$k_e$ [kip/in] |
|----------|-----------------------------------|---------------------------------------|--------------------------------------|
| SGGS-1   | 36.0                              | 2.11 (1.95%)                          | 47.4                                 |
| SGGS-1F  | 46.8 (↑30%)                       | 2.05 (1.90%)                          | 116.8 (↑150%)                        |



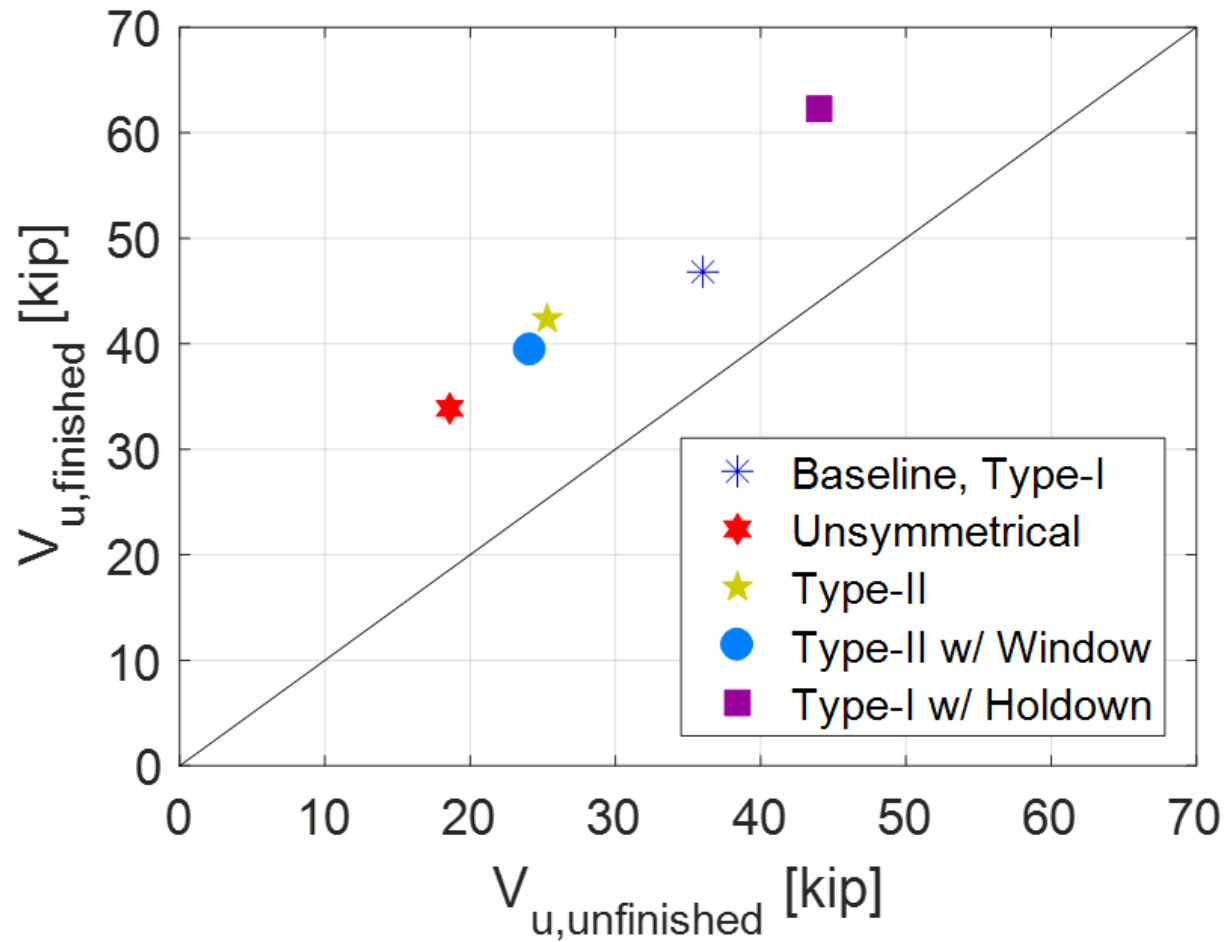
- **Strength** increase
- **Initial stiffness** increase
- No negative effect on **drift at strength**

Unfinished = Wall framing

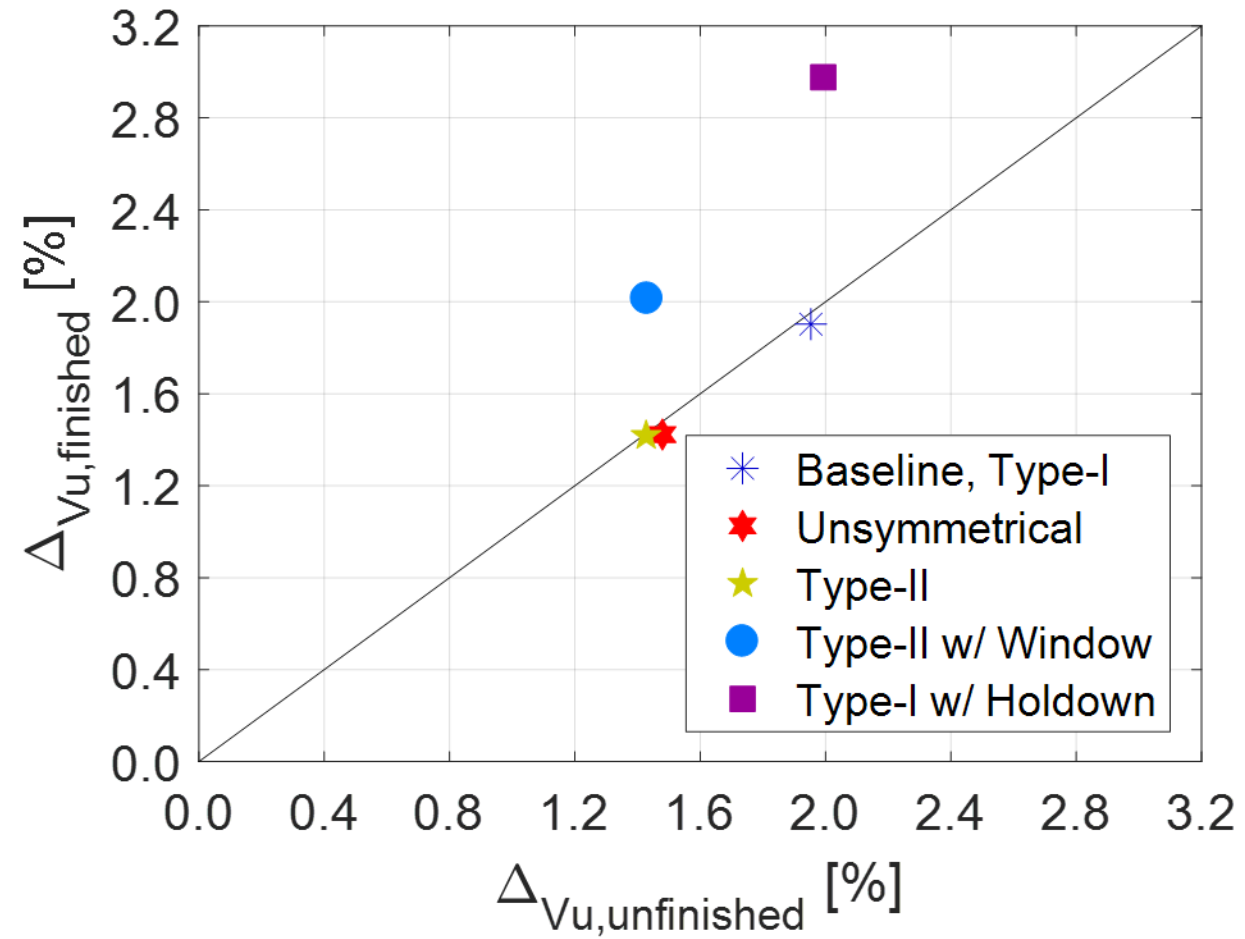
Finished = Exterior Insulation Finish System (EIFS) and gypsum panels on interior face on wall framing



# Effect of Finish Application

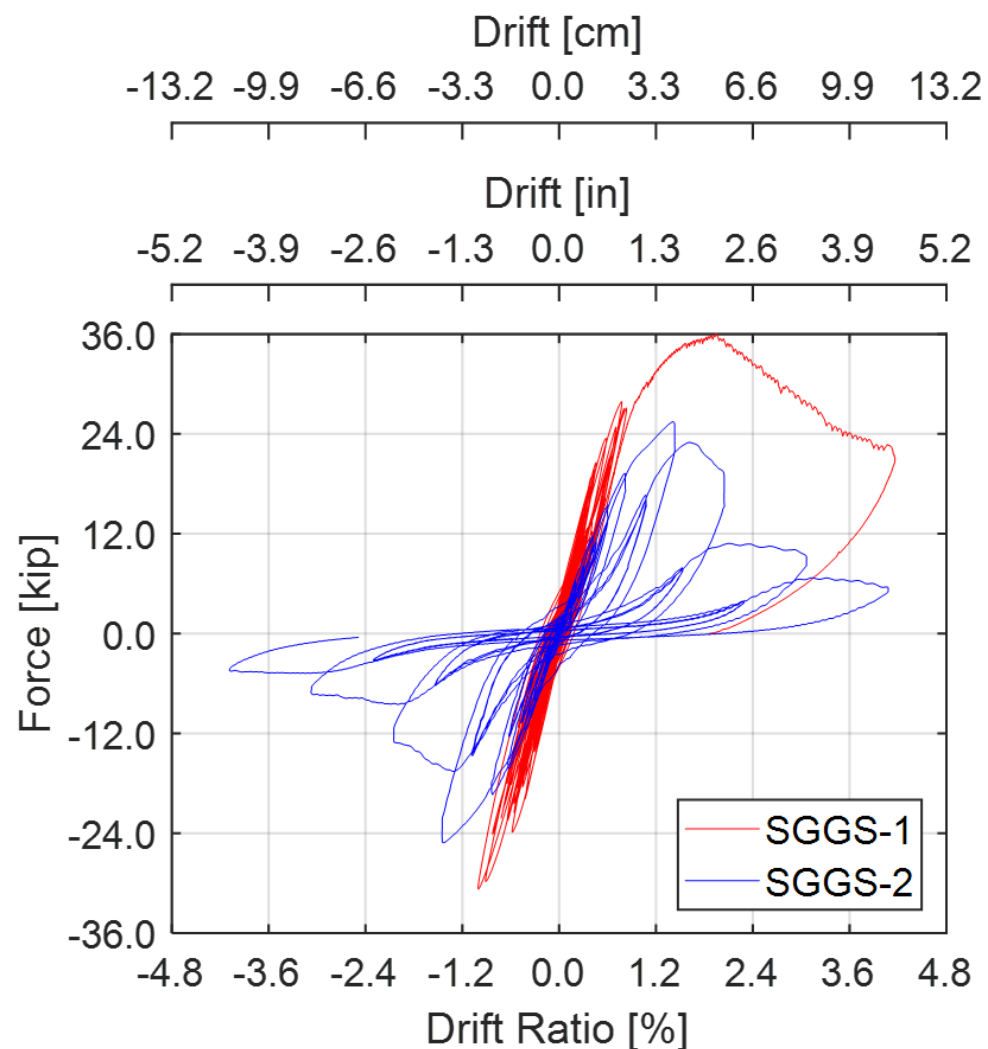


$$V_{u,finished} = 1.01 \times V_{u,unfinished} + 930 \text{ plf}$$



- **Strength** increase (additive strength model)
- No negative effect on **drift at strength**

# Comparison: Type-I vs Type-II Wall Systems

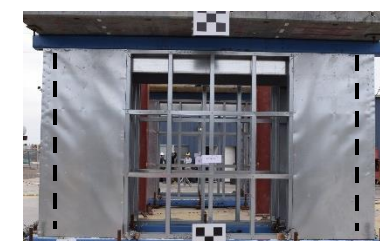


| Specimen | Strength, $V_{max}$ [kip] | Drift, $\delta_{V_{max}}$ [in] (%) | Initial Stiffness*, $k_e$ [kip/in] |
|----------|---------------------------|------------------------------------|------------------------------------|
| SGGS-1   | 36.0                      | 2.11 (1.95%)                       | 47.4                               |
| SGGS-2   | 25.5 (↓30%)               | 1.53 (1.41%)                       | 25.9 (↓45%)                        |

- **Strength** decrease, but not 50% as suggested by code
- **Initial stiffness** decrease
- Lower **drift at strength**



SGGS-1



SGGS-2

Type-I = anchorage at each end of wall segment

Type-II = anchorage at ends of wall

# Concluding Remarks: Observations

- Symmetry: Unsymmetrical wall with one 4ft shear wall segment
  - ~50% ↓ in strength and stiffness
  - Aligning with conceptual design perspective
- Window Opening & Window Framing:
  - Negligible effect on strength or stiffness
  - Drift capacity not effected
  - Damage to adjacent stud packs concerning
- Tension Tie-Rod/Holdown detailing:
  - Strength ↓ 20%-30% with holdowns
  - Similar initial stiffness
  - Tension rods offer advantages: easy installation and continuous floor-to-floor system
- Finishes: EIFS and Gypsum boards
  - Strength ↓ 30%-80% = 930plf
  - Initial stiffness ↓ 1.5x-3x
  - Period elongation < 10% (QE)
  - Damping ~ ↓ 50%
  - No derogatory effect on drift capacity
- Anchorage Detailing: Type-II wall
  - ~30%-35% ↓ in strength, not 50% as suggested by code
  - Initial stiffness ↓ 30%-45% with tie-rods anchorage at ends of wall





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